

INTERIOR WALL HEIGHTS

With structural framing

Member	Spacing (in) o.c.	5psf		
		L/120	L/240	L/360
362S137-33	12	23'3"	18'5"	16'1"
	16	21'1"	16'9"	14'8"
	24	17'6"	14'8"	12'10"
362S137-43	12	25'3"	20'1"	17'6"
	16	23'0"	18'3"	15'11"
	24	20'1"	15'11"	13'11"
362S137-54	12	21'6"	18'9"	17'1"
	16	24'7"	19'6"	17'1"
	24	21'6"	17'1"	14'11"
362S137-68	12	28'11"	22'11"	20'1"
	16	26'3"	20'10"	18'3"
	24	22'11"	18'3"	15'11"
362S137-97	12	31'10"	25'3"	22'1"
	16	28'11"	22'11"	20'1"
	24	25'3"	20'1"	17'6"
362S162-33	12	24'4"	19'4"	16'11"
	16	22'2"	17'7"	15'4"
	24	18'9"	15'4"	13'5"
362S162-43	12	26'6"	21'0"	18'5"
	16	24'1"	19'1"	16'8"
	24	21'0"	16'8"	14'7"
362S162-54	12	28'5"	22'6"	19'8"
	16	25'10"	20'6"	17'11"
	24	22'6"	17'11"	15'7"
362S162-68	12	30'5"	24'1"	21'1"
	16	27'7"	21'11"	19'2"
	24	24'1"	19'2"	16'9"
362S162-97	12	33'6"	26'7"	23'3"
	16	30'5"	24'2"	21'1"
	24	26'7"	21'1"	18'5"

3-5/8" Structural Framing

Member	Spacing (in) o.c.	5psf		
		L/120	L/240	L/360
362S200-33	12	25'8"	20'4"	17'9"
	16	23'3"	18'6"	16'2"
	24	19'8"	16'2"	14'1"
362S200-43	12	28'0"	22'3"	19'5"
	16	25'5"	20'2"	17'8"
	24	22'3"	17'8"	15'5"
362S200-54	12	30'0"	23'10"	20'10"
	16	27'3"	21'8"	18'11"
	24	23'10"	18'11"	16'6"
362S200-68	12	32'2"	25'6"	22'3"
	16	29'2"	23'2"	20'3"
	24	25'6"	20'3"	17'8"
362S200-97	12	35'6"	28'3"	24'8"
	16	32'3"	25'8"	22'5"
	24	28'3"	22'5"	19'7"
362S250-43	12	29'6"	23'5"	20'6"
	16	26'10"	21'3"	18'7"
	24	23'5"	18'7"	16'3"
362S250-54	12	31'7"	25'1"	21'11"
	16	28'8"	22'9"	19'11"
	24	25'1"	19'11"	17'4"
362S250-68	12	33'11"	26'11"	23'6"
	16	30'10"	24'6"	21'5"
	24	26'11"	21'5"	18'8"
362S250-97	12	37'7"	29'10"	26'1"
	16	34'2"	27'1"	23'8"
	24	29'10"	23'8"	20'8"

3-5/8" Structural Framing

Notes:

- 1 Studs are checked for simple-span deflection and stress. Stress calculations are made for mid-span fully braced moment, end shear through the unperforated section, and shear moment interaction through the perforated section 10" away from the end bearing.
- 2 A 1/3 stress increase is not used.
- 3 Limiting heights are based on continuous lateral support of each flange over the full height of the stud.
- 4 Listed limiting heights are based on steel properties only.
- 5 Web crippling check based on 1-inch end bearing. Where limiting heights are followed by "e", web stiffeners are required.
- 6 Allowable moment is the lesser of local and distortional buckling. Stud distortional buckling based on an assumed  $K_p=0$ .
- 7 Cells marked with an " \* " have  $h/t > 200$ , and thus require end stiffeners.
- 8 Capacities are calculated according to the AISI-NASPEC S100-2007, with 2010 supplement. A 1-1/2" by 4" knockout spaced no closer than 24" o.c. is assumed. (3/4" by 4" for 2-1/2" studs.)
- 9 All values are based on  $F_y=33\text{ksi}$  for 33mil and 43mil studs, and  $F_y=50\text{ksi}$  for 54mil, 68mil and 97mil studs.
- 10 For deflection calculations, interior wall loads have been multiplied by 1.0 per the AISI S211-07 Standard for Cold-Formed Steel Framing—Wall Stud Design 2007 Edition.

# INTERIOR WALL HEIGHTS

With structural framing

Member	Spacing (in) o.c.	5psf		
		L/120	L/240	L/360
600S137-33	12	33' 1"	27' 3"	23' 10"
	16	28' 7"	24' 9"	21' 8"
	24	23' 4"	21' 8"	18' 11"
600S137-43	12	37' 8"	29' 11"	26' 2"
	16	34' 3"	27' 2"	23' 9"
	24	28' 1"	23' 9"	20' 9"
600S137-54	12	40' 5"	32' 1"	28' 0"
	16	36' 9"	29' 2"	25' 6"
	24	32' 1"	25' 6"	22' 3"
600S137-68	12	43' 4"	34' 4"	30' 0"
	16	39' 4"	31' 3"	27' 3"
	24	34' 4"	27' 3"	23' 10"
600S137-97	12	47' 11"	38' 0"	33' 2"
	16	43' 6"	34' 6"	30' 2"
	24	38' 0"	30' 2"	26' 4"
600S162-33	12	35' 6"	28' 8"	25' 0"
	16	30' 9"	26' 0"	22' 9"
	24	25' 2"	22' 9"	19' 10"
600S162-43	12	39' 4"	31' 2"	27' 3"
	16	35' 9"	28' 4"	24' 9"
	24	31' 1"	24' 9"	21' 8"
600S162-54	12	42' 2"	33' 6"	29' 3"
	16	38' 4"	30' 5"	26' 7"
	24	33' 6"	26' 7"	23' 3"
600S162-68	12	45' 3"	35' 11"	31' 4"
	16	41' 1"	32' 7"	28' 6"
	24	35' 11"	28' 6"	24' 11"
600S162-97	12	50' 1"	39' 9"	34' 9"
	16	45' 6"	36' 2"	31' 7"
	24	39' 9"	31' 7"	27' 7"

6" Structural Framing

Member	Spacing (in) o.c.	5psf		
		L/120	L/240	L/360
600S200-33	12	37' 9"	30' 0"	26' 2"
	16	27' 3"	23' 10"	20' 10"
	24	26' 10"	23' 10"	20' 10"
600S200-43	12	41' 3"	32' 9"	28' 7"
	16	37' 6"	29' 9"	26' 0"
	24	32' 0"	26' 0"	22' 9"
600S200-54	12	44' 4"	35' 2"	30' 9"
	16	40' 3"	32' 0"	27' 11"
	24	35' 2"	27' 11"	24' 5"
600S200-68	12	47' 7"	37' 9"	33' 0"
	16	43' 2"	34' 4"	29' 11"
	24	37' 9"	29' 11"	26' 2"
600S200-97	12	52' 10"	41' 11"	36' 7"
	16	48' 0"	38' 1"	33' 3"
	24	41' 11"	33' 3"	29' 1"
600S250-43	12	43' 3"	34' 4"	30' 0"
	16	39' 3"	31' 2"	27' 3"
	24	32' 11"	27' 3"	23' 10"
600S250-54	12	46' 3"	36' 8"	32' 1"
	16	42' 0"	33' 4"	29' 1"
	24	36' 8"	29' 1"	25' 5"
600S250-68	12	49' 10"	39' 7"	34' 7"
	16	45' 3"	35' 11"	31' 5"
	24	39' 7"	31' 5"	27' 5"
600S250-97	12	55' 5"	44' 0"	38' 5"
	16	50' 4"	40' 0"	34' 11"
	24	44' 0"	34' 11"	30' 6"

6" Structural Framing

## Notes:

- 1 Studs are checked for simple-span deflection and stress. Stress calculations are made for mid-span fully braced moment, end shear through the unperforated section, and shear moment interaction through the perforated section 10" away from the end bearing.
- 2 A 1/3 stress increase is not used.
- 3 Limiting heights are based on continuous lateral support of each flange over the full height of the stud.
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- 6 Allowable moment is the lesser of local and distortional buckling. Stud distortional buckling based on an assumed  $K\phi=0$ .
- 7 Cells marked with an "x" have  $h/t > 200$ , and thus require end stiffeners.
- 8 Capacities are calculated according to the AISI-NASPEC S100-2007, with 2010 supplement. A 1-1/2" by 4" knockout spaced no closer than 24" o.c. is assumed. (3/4" by 4" for 2-1/2" studs.)
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- 10 For deflection calculations, interior wall loads have been multiplied by 1.0 per the AISI S211-07 Standard for Cold-Formed Steel Framing—Wall Stud Design 2007 Edition.